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A METHOD FOR ESTIMATING THE PROPERTIES OF A  
SOLID MATERIAL SUBJECTED TO COMPRESSIONAL FORCES

TO ALL WHOM IT MAY CONCERN:

BE IT KNOWN THAT ANDREW J. HULL, citizen of the United States of America, employee of the United States Government and resident Newport, County of Newport, State of Rhode Island, has invented certain new and useful improvements entitles as set forth above of which the following is a specification:

JAMES M. KASISCHKE ESQ.  
Reg. No. 36562  
Naval Undersea Warfare Center  
Division Newport  
Newport, RI 02841-1708  
TEL: 401-832-4736  
FAX: 401-832-1231

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James M. Kasischke  
APPLICANT'S ATTORNEY

9 Feb 2004  
DATE OF SIGNATURE

2  
3 A METHOD FOR ESTIMATING THE PROPERTIES OF A  
4 SOLID MATERIAL SUBJECTED TO COMPRESSIONAL FORCES

5  
6 STATEMENT OF GOVERNMENT INTEREST

7 The invention described herein may be manufactured and used  
8 by or for the Government of the United States of America for  
9 governmental purposes without the payment of any royalties  
10 thereon or therefore.  
11

12 BACKGROUND OF THE INVENTION

13 (1) Field of the Invention

14 The present invention relates to a method to measure (or  
15 estimate) the complex frequency-dependent dilatational and shear  
16 wavenumbers of a single slab of material subjected to large  
17 static compressional forces. More particularly, this invention  
18 provides a method to determine complex dilatational wavespeed,  
19 complex shear wavespeed, complex Lamé constants, complex Young's  
20 modulus, complex shear modulus, and complex Poisson's ratio.

21 (2) Description of the Prior Art

22 Measuring the mechanical properties of slab-shaped materials  
23 are important because these parameters significantly contribute  
24 to the static and dynamic response of structures built with such  
25 materials. One characteristic that most elastomeric solids

1 possess is that, when they are subjected to large static forces  
2 (or pressure), their rigidity changes. Materials that have one  
3 set of mechanical properties at a pressure of one atmosphere can  
4 have very different properties when subjected to increased  
5 pressure. The ability to determine the pressure dependence of  
6 material properties is extremely important for modeling the  
7 behavior of systems comprised of these materials.

8 Resonant techniques have been used to identify and measure  
9 longitudinal and shear properties for many years. These methods  
10 are based on comparing measured eigenvalues to modeled  
11 eigenvalues and calculating the resulting material properties.  
12 These methods do not account for static pressure or large  
13 compressive forces. Additionally, they typically require long,  
14 slender materials to perform the measurement process. Comparison  
15 of analytical models to measured frequency response functions are  
16 also used to estimate stiffness and loss parameters of a  
17 structure. When the analytical model agrees with one or more  
18 frequency response functions, the parameters used to calculate  
19 the analytical model are considered accurate. If the analytical  
20 model is formulated using a numerical method, a comparison of the  
21 model to the data can be difficult due to dispersion properties  
22 of the materials. These methods do not take into account large  
23 compressive forces.

24 In the prior art, some efforts have been made to measure  
25 material properties under large pressures. These methods consist

1 of placing materials in pressurized settings, insonifying them,  
2 and then measuring their response. These methods are difficult  
3 because they have to be conducted under great atmospheric  
4 pressure that can adversely affect the instrumentation. Safety  
5 issues can also arise in connection with laboratory testing at  
6 extreme pressures. Finally, a mass loaded long thin rod has been  
7 studied with respect to the bar wavespeed and corresponding  
8 Young's modulus; however, this work does not investigate shear  
9 motion.

10 Accordingly, there is a need for a method of measuring  
11 mechanical properties of slab-shaped materials placed under  
12 pressure.

#### 14 SUMMARY OF THE INVENTION

15 Accordingly, in this invention, a method to measure the  
16 complex frequency-dependent dilatational and shear wavenumbers of  
17 a material under a static compressional force is provided. The  
18 material is first vibrated in both vertical and horizontal  
19 directions while obtaining transfer functions in each direction.  
20 The two transfer functions are combined with a theoretical model  
21 to estimate a dilatational wavenumber and a shear wavenumber.  
22 The wavenumbers can be combined to give the complex dilatational  
23 wavespeed, complex shear wavespeed, complex Lamé constants,  
24 complex Young's modulus, complex shear modulus, and complex  
25 Poisson's ratio.

1 BRIEF DESCRIPTION OF THE DRAWINGS.

2 A more complete understanding of the invention and many of  
3 the attendant advantages thereto will be readily appreciated as  
4 the same becomes better understood by reference to the following  
5 detailed description when considered in conjunction with the  
6 accompanying drawings wherein:

7 FIG. 1 shows apparatus for measurement of transfer functions  
8 in a vertical direction according to the current invention;

9 FIG. 2 shows apparatus for measurement of transfer functions  
10 in a horizontal direction according to the current invention;

11 FIG. 3 is a diagram of the coordinate system of used with a  
12 test specimen in the model;

13 FIG. 4A is a plot of the transfer function magnitude versus  
14 input frequency for the vertical direction test;

15 FIG. 4B is a plot of the transfer function phase angle  
16 versus input frequency for the vertical direction test;

17 FIG. 5A is a plot of the transfer function magnitude versus  
18 input frequency for the horizontal direction test;

19 FIG. 5B is a plot of the transfer function phase angle  
20 versus input frequency for the horizontal direction test;

21 FIG. 6 is a contour plot of the absolute value of the  
22 dilatational wavenumber on an real-imaginary coordinate system of  
23 the dilatational wavenumbers at 2000 Hz;

FIG. 7 is a contour plot of the absolute value of the dilatational wavenumber on an real-imaginary coordinate system of the dilatational wavenumbers at 5000 Hz;

FIG. 8A is a plot of the real dilatational wavenumber versus frequency;

FIG. 8B is a plot of the imaginary dilatational wavenumber versus frequency; and

FIG. 9A is a plot of the real shear wavenumber versus frequency;

FIG. 9B is a plot of the imaginary shear wavenumber versus frequency;

FIG. 10 is a plot of the real and imaginary Young's modulus versus frequency;

FIG. 11 is a plot of the real and imaginary shear modulus versus frequency; and

FIG. 12 is a plot of the Poisson's ratio versus frequency.

#### DESCRIPTION OF THE PREFERRED EMBODIMENT

The test procedure consists of vibrating a mass-loaded, slab-shaped test specimen 10 with a shaker 12 in two different directions, vertical 14A and horizontal 14B, as shown in FIGS. 1 and 2, respectively. It is noted that the load mass 16 attached to the top of the test specimen 10 must be sufficiently stiffer than the specimen 10 that it can be modeled as lumped parameter expression rather than a continuous media system. A typical

1 example would be a steel load mass 16 attached above a rubber-  
2 like material test specimen 10. This example results in a ratio  
3 between the two stiffnesses of greater than 100. Lower ratios  
4 result in less accurate estimations. Vibrating the combined  
5 specimen 10 and load mass 16 causes different waveforms to  
6 propagate in the specimen 10. The inverse method developed here  
7 allows for the data from the experiments to be manipulated so  
8 that the complex dilatational and shear wavenumbers can be  
9 measured for the specimen 10. This test is usually done at  
10 multiple frequencies (swept sine) so any frequency dependencies  
11 can be identified and measured. Input vibration data is  
12 collected from the shaker 12. A sensor 18 is mounted on load  
13 mass 16 and another sensor 20 is mounted on shaker 12 for  
14 collecting transfer function data. In FIG. 1, the test is set up  
15 for monitoring the vertical transfer function. FIG. 2 shows the  
16 test as set up for monitoring the horizontal transfer function.  
17 Sensors 18 and 20 should be oriented properly to capture the  
18 motion being measured. Other test configurations using  
19 directions other than vertical and horizontal are possible;  
20 however, the test setups shown are preferred for ease of set up  
21 and calculation. These sensors 18 can be either accelerometers  
22 that record accelerations, or laser velocimeters that record  
23 velocities. In the swept sine mode, transfer functions of  
24 acceleration divided by acceleration or velocity divided by  
25 velocity are both equal to displacement divided by displacement.

1 The time domain data collected from the sensors 18 and 20 are  
2 Fourier transformed into the frequency domain and then recorded  
3 as complex transfer functions, typically using a spectrum  
4 analyzer 22.

5 The motion of the test specimen shown in FIGS. 1 and 2 is  
6 governed by the equation:

7

$$8 \quad \mu \nabla^2 \mathbf{u} + (\lambda + \mu) \nabla \nabla \cdot \mathbf{u} = \rho \frac{\partial^2 \mathbf{u}}{\partial t^2} , \quad (1)$$

9

10 where  $\lambda$  and  $\mu$  are the complex Lamé constants ( $\text{N/m}^2$ );

11  $\rho$  is the density ( $\text{kg/m}^3$ );

12  $t$  is time (s);

13  $\cdot$  denotes a vector dot product; and

14  $\mathbf{u}$  is the Cartesian coordinate displacement vector of the  
15 material.

16 The coordinate system of the test configuration is shown in  
17 FIG. 3. Note that using this orientation results in  $b = 0$  and  $a$   
18 having a value less than zero. The thickness of the specimen,  $h$ ,  
19 is a positive value. Equation (1) is manipulated by writing the  
20 displacement vector  $\mathbf{u}$  as

21

$$22 \quad \mathbf{u} = \begin{Bmatrix} u_x(x,y,z,t) \\ u_y(x,y,z,t) \\ u_z(x,y,z,t) \end{Bmatrix} , \quad (2)$$



where  $x$  is the location along the plate (m),  $y$  is the location into the plate (m), and  $z$  is the location normal to the plate (m), as shown in FIG. 3. The symbol  $\nabla$  is the gradient vector differential operator written in three-dimensional Cartesian coordinates as

$$\nabla = \frac{\partial}{\partial x} i_x + \frac{\partial}{\partial y} i_y + \frac{\partial}{\partial z} i_z \quad , \quad (3)$$

with  $i_x$  denoting the unit vector in the  $x$ -direction,  $i_y$  denoting the unit vector in the  $y$ -direction, and  $i_z$  denoting the unit vector in the  $z$ -direction;  $\nabla^2$  is the three-dimensional Laplace operator operating on vector  $\mathbf{u}$  as

$$\nabla^2 \mathbf{u} = \nabla^2 u_x i_x + \nabla^2 u_y i_y + \nabla^2 u_z i_z \quad , \quad (4)$$

and operating on scalar  $u$  as

$$\nabla^2 u_{x,y,z} = \nabla \cdot \nabla u_{x,y,z} = \frac{\partial^2 u_{x,y,z}}{\partial x^2} + \frac{\partial^2 u_{x,y,z}}{\partial y^2} + \frac{\partial^2 u_{x,y,z}}{\partial z^2} \quad ; \quad (5)$$

and the term  $\nabla \cdot \mathbf{u}$  is called the divergence and is equal to

$$\nabla \cdot \mathbf{u} = \frac{\partial u_x}{\partial x} + \frac{\partial u_y}{\partial y} + \frac{\partial u_z}{\partial z} \quad . \quad (6)$$

1  
2 The displacement vector  $\mathbf{u}$  is written as

3  
4 
$$\mathbf{u} = \nabla\phi + \nabla \times \bar{\psi} , \quad (7)$$

5  
6 where  $\phi$  is a dilatational scalar potential,  $\times$  denotes a vector  
7 cross product, and  $\bar{\psi}$  is an equivoluminal vector potential  
8 expressed as

9  
10 
$$\bar{\psi} = \begin{Bmatrix} \psi_x(x,y,z,t) \\ \psi_y(x,y,z,t) \\ \psi_z(x,y,z,t) \end{Bmatrix} . \quad (8)$$

11  
12 The problem is formulated as a two-dimensional system, thus  $y=0$ ,  
13  $u_y(x,y,z,t) \equiv 0$ , and  $\partial(\cdot)/\partial y \equiv 0$ . Expanding equation (7) and breaking  
14 the displacement vector into its individual nonzero terms yields

15  
16 
$$u_x(x,z,t) = \frac{\partial\phi(x,z,t)}{\partial x} - \frac{\partial\psi_y(x,z,t)}{\partial z} \quad (9)$$

17  
18 and

19  
20 
$$u_z(x,z,t) = \frac{\partial\phi(x,z,t)}{\partial z} + \frac{\partial\psi_y(x,z,t)}{\partial x} \quad (10)$$

Equations (9) and (10) are next inserted into equation (1),  
which results in

$$c_d^2 \nabla^2 \phi(x, z, t) = \frac{\partial^2 \phi(x, z, t)}{\partial t^2} \quad (11)$$

and

$$c_s^2 \nabla^2 \psi_y(x, z, t) = \frac{\partial^2 \psi_y(x, z, t)}{\partial t^2} \quad (12)$$

where equation (11) corresponds to the dilatational component and  
equation (12) corresponds to the shear component of the  
displacement field. Correspondingly, the constants  $c_d$  and  $c_s$  are  
the complex dilatational and shear wave speeds, respectively, and  
are determined by

$$c_d = \sqrt{\frac{\lambda + 2\mu}{\rho}} \quad (13)$$

and

$$c_s = \sqrt{\frac{\mu}{\rho}} \quad (14)$$

1 The relationship of the Lamé constants to the Young's and shear  
2 moduli is shown as

3

$$4 \quad \lambda = \frac{E\nu}{(1+\nu)(1-2\nu)} \quad (15)$$

5  
6 and

7

$$8 \quad \mu = G = \frac{E}{2(1+\nu)} \quad (16)$$

9

10 where  $E$  is the complex Young's modulus ( $\text{N/m}^2$ ),  $G$  is the complex  
11 shear modulus ( $\text{N/m}^2$ ), and  $\nu$  is the Poisson's ratio of the  
12 material (dimensionless).

13 The conditions of infinite length and steady-state response  
14 are now imposed, allowing the scalar and vector potential to be  
15 written as

16

$$17 \quad \phi(x, z, t) = \Phi(z) \exp(ikx) \exp(i\omega t) \quad (17)$$

18  
19 and

20

$$21 \quad \psi_y(x, z, t) = \Psi(z) \exp(ikx) \exp(i\omega t) \quad (18)$$

1 where  $i$  is the square root of  $-1$ ,  $\omega$  is frequency (rad/s), and  $k$   
 2 is wavenumber with respect to the  $x$  axis (rad/m). Inserting  
 3 equation (17) into equation (11) yields

$$4 \quad \frac{d^2 \Phi(z)}{dz^2} + \alpha^2 \Phi(z) = 0, \quad (19)$$

6  
 7 where

$$8 \quad \alpha = \sqrt{k_d^2 - k^2}, \quad (20)$$

10  
 11 and

$$12 \quad k_d = \frac{\omega}{c_d}, \quad (21)$$

14  
 15 Inserting equation (18) into equation (12) yields

$$16 \quad \frac{d^2 \Psi(z)}{dz^2} + \beta^2 \Psi(z) = 0, \quad (22)$$

18  
 19 where

$$20 \quad \beta = \sqrt{k_s^2 - k^2}, \quad (23)$$

1 and

2

$$3 \quad k_s = \frac{\omega}{c_s} \quad (24)$$

4

5 The solution to equation (19) is

6

$$7 \quad \Phi(z) = A(k, \omega) \exp(i\alpha z) + B(k, \omega) \exp(-i\alpha z) \quad (25)$$

8

9 and the solution to equation (22) is

10

$$11 \quad \Psi(z) = C(k, \omega) \exp(i\beta z) + D(k, \omega) \exp(-i\beta z) \quad (26)$$

12

13 where  $A(k, \omega)$ ,  $B(k, \omega)$ ,  $C(k, \omega)$ , and  $D(k, \omega)$  are wave response

14 coefficients that are determined below. The displacements can

15 now be written as functions of the unknown constants using the

16 expressions in equations (9) and (10). They are

17

$$u_z(x, z, t) = U_z(k, z, \omega) \exp(ikx) \exp(i\omega t)$$

$$18 \quad = \{ i\alpha [A(k, \omega) \exp(i\alpha z) - B(k, \omega) \exp(-i\alpha z)] + \quad (27)$$

$$ik [C(k, \omega) \exp(i\beta z) + D(k, \omega) \exp(-i\beta z)] \} \exp(ikx) \exp(i\omega t) ,$$

19

20 and

$$u_x(x,z,t) = U_x(k,z,\omega) \exp(ikx) \exp(i\omega t)$$

$$= \{ ik[A(k,\omega)\exp(i\alpha z) + B(k,\omega)\exp(-i\alpha z)] - i\beta[C(k,\omega)\exp(i\beta z) - D(k,\omega)\exp(-i\beta z)] \} \exp(ikx) \exp(i\omega t) \quad (28)$$

Specific boundary conditions are now needed to individually solve for the case of vertical and horizontal motion. These are formulated separately below.

For the case of vertical motion, the base at  $z = a$  is vibrated vertically using a shaker, as shown in FIG. 1. Four boundary conditions are necessary to formulate this problem. Because the mass is attached to the material, the tangential (horizontal) motion at the top of the plate ( $z = b$ ) is zero and this equation is written as

$$u_x(x,b,t) = 0 \quad (29)$$

The normal stress at the top of the specimen is equal to and opposite the load created by the mass in the  $z$  direction. This expression is

$$\tau_{zz}(x,b,t) = (\lambda + 2\mu) \frac{\partial u_z(x,b,t)}{\partial z} + \lambda \frac{\partial u_x(x,b,t)}{\partial x} = -M \frac{\partial^2 u_z(x,b,t)}{\partial t^2} \quad (30)$$

1 where  $M$  is mass per unit area ( $\text{kg/m}^2$ ) of the attached mass. The  
2 tangential motion at the bottom of the plate ( $z = a$ ) is zero and  
3 this equation is written as

$$4 \quad u_x(x, a, t) = 0, \quad (31)$$

6  
7 and the normal motion at the bottom of the plate is prescribed as  
8 a system input. This expression is

$$9 \quad u_z(x, a, t) = U_0 \exp(i\omega t). \quad (32)$$

11  
12 Assembling equations (1) - (32) and letting  $b = 0$  yields the  
13 four-by-four system of linear equations that model the system.  
14 They are

$$15 \quad \mathbf{Ax} = \mathbf{b}, \quad (33)$$

17  
18 where the entries of equation (33) are

$$19 \quad A_{11} = ik, \quad (34)$$

$$20 \quad A_{12} = A_{11}, \quad (35)$$

$$21 \quad A_{13} = -i\beta, \quad (36)$$



$$1 \quad A_{14} = -A_{13} \quad , \quad (37)$$

2

$$3 \quad A_{21} = -\alpha^2 \lambda - 2\alpha^2 \mu - \lambda k^2 - iM\omega^2 \alpha \quad , \quad (38)$$

4

$$5 \quad A_{22} = -\alpha^2 \lambda - 2\alpha^2 \mu - \lambda k^2 + iM\omega^2 \alpha \quad , \quad (39)$$

6

$$7 \quad A_{23} = -2k\beta\mu - iM\omega^2 k \quad , \quad (40)$$

8

$$9 \quad A_{24} = 2k\beta\mu - iM\omega^2 k \quad , \quad (41)$$

10

$$11 \quad A_{31} = A_{11} \exp(i\alpha a) \quad , \quad (42)$$

12

$$13 \quad A_{32} = A_{11} \exp(-i\alpha a) \quad , \quad (43)$$

14

$$15 \quad A_{33} = A_{13} \exp(i\beta a) \quad , \quad (44)$$

16

$$17 \quad A_{34} = -A_{13} \exp(-i\beta a) \quad , \quad (45)$$

18

$$19 \quad A_{41} = i\alpha \exp(i\alpha a) \quad , \quad (46)$$

20

$$21 \quad A_{42} = -i\alpha \exp(-i\alpha a) \quad , \quad (47)$$

22

$$23 \quad A_{43} = ik \exp(i\beta a) \quad , \quad (48)$$

$$1 \quad A_{44} = ik \exp(-i\beta a) \quad , \quad (49)$$

$$2 \quad$$

$$3 \quad x_{11} = A(k, \omega) \quad , \quad (50)$$

$$4 \quad$$

$$5 \quad x_{21} = B(k, \omega) \quad , \quad (51)$$

$$6 \quad$$

$$7 \quad x_{31} = C(k, \omega) \quad , \quad (52)$$

$$8 \quad$$

$$9 \quad x_{41} = D(k, \omega) \quad , \quad (53)$$

$$10 \quad$$

$$11 \quad b_{11} = 0 \quad , \quad (54)$$

$$12 \quad$$

$$13 \quad b_{21} = 0 \quad , \quad (55)$$

$$14 \quad$$

$$15 \quad b_{31} = 0 \quad , \quad (56)$$

16 and

$$17 \quad$$

$$18 \quad b_{41} = U_0 \quad . \quad (57)$$

19

20 Using equations (34) - (57) the solution to the constants  $A(k, \omega)$ ,

21  $B(k, \omega)$ ,  $C(k, \omega)$ , and  $D(k, \omega)$  can be calculated at each specific

22 wavenumber and frequency using

$$23 \quad$$

$$24 \quad \mathbf{x} = \mathbf{A}^{-1} \mathbf{b} \quad . \quad (58)$$

Noting that for vertical motion,  $k = 0$ , and using the coefficients from equation (58), the transfer function between the vertical base displacement and the vertical mass displacement can be written as

$$T_1(\omega) = \frac{1}{R_1(\omega)} = \frac{U_z(0, b, \omega)}{U_0} = \frac{1}{\cos(k_d h) - \left(\frac{M}{\rho}\right) k_d \sin(k_d h)} \quad (59)$$

where  $T_1(\omega)$  or  $R_1(\omega)$  correspond to the data from the vertical motion experiment.

The next step is to solve the inverse problem for vertical motion. This involves using the experimental data and equation (59) to estimate the dilatational wavenumber. Equation (59) can be rewritten as

$$f(k_d) = 0 = \cos(k_d h) - \left(\frac{M}{\rho}\right) k_d \sin(k_d h) - R_1 \quad (60)$$

where the problem now becomes finding the zeros of the right-hand side of equation (60), or, in the presence of actual data that contains noise, finding the relative minima of the right-hand side of equation (60) and determining which relative minimum corresponds to dilatational wave propagation and which relative minima are extraneous. Because equation (60) has a number of relative minima, zero finding algorithms are not applied to this

1 function, as they typically do not find all of the minima  
2 locations and are highly dependent on initial starting locations.  
3 The best method to find all of the minima locations is by  
4 plotting the absolute value of the right-hand side of equation  
5 (60) as a surface with the real part of dilatational wavenumber  
6  $k_d$  on one axis and the imaginary part of  $k_d$  on the other axis.  
7 In order to do this, the user should start at a low frequency  
8 where the aliasing minimum has not yet appeared. In the specific  
9 example shown herein, this is below 3850 Hz for the dilatational  
10 wave and below 1550 Hz for the shear wave. At these lower  
11 frequencies, the minimum furthest to the left will correspond to  
12 dilatational wave propagation. As the frequency increases,  
13 extraneous minima will appear to the left of the minimum that  
14 corresponds to dilatational wave propagation, however, the wave  
15 propagation minimum will always be close to the previous test  
16 frequency wave propagation minimum provided that the frequency  
17 increments are relatively small. At a resolution of 0.5 rad/m  
18 for the materials in the example herein, this requires a  
19 frequency increment of 37.3 Hz for the dilatational measurement  
20 and 14.4 Hz for the shear measurement. Different test specimens  
21 and top masses require different increments. Additionally, the  
22 real part of the wavenumber is a monotonically increasing  
23 function with respect to frequency, so at each increase in  
24 frequency, the new wavenumber to be estimated has to be greater  
25 than the old wavenumber that was previously estimated. This

1 process is further illustrated as related to the discussion  
2 concerning FIG. 6 and FIG. 7 below.

3 For the case of horizontal motion, the base at  $z = a$  is  
4 vibrated horizontally using a shaker, as shown in FIG. 2. Four  
5 boundary conditions are necessary to formulate this problem.  
6 Because the mass is attached to the material, the shear  
7 (tangential) stress at the top of the plate is equal to opposite  
8 the load created by the mass in the  $x$  direction. This expression  
9 is

10

$$11 \quad \tau_{zx}(x,b,t) = \mu \left[ \frac{\partial u_x(x,b,t)}{\partial z} + \frac{\partial u_z(x,b,t)}{\partial x} \right] = -M \frac{\partial^2 u_x(x,b,t)}{\partial t^2} , \quad (61)$$

12

13 where  $M$  is mass per unit area ( $\text{kg/m}^2$ ) of the attached mass. The  
14 normal motion at the top of the plate ( $z = b$ ) is zero and this  
15 equation is written as

16

$$17 \quad u_z(x,b,t) = 0 . \quad (62)$$

18

19 The tangential motion at the bottom of the plate ( $z = a$ ) is  
20 prescribed as a system input and this equation is written as

21

$$22 \quad u_x(x,a,t) = V_0 \exp(i\omega t) , \quad (63)$$

1 and the normal motion at the bottom of the plate is zero. This  
2 expression is

3

$$4 \quad u_z(x, a, t) = 0 \quad (64)$$

5

6 Assembling equations (1) - (28) and (62) - (64) and letting  
7  $b = 0$  yields the four-by-four system of linear equations that  
8 model the system. They are

9

$$10 \quad \mathbf{Ax} = \mathbf{b} \quad (65)$$

11

12 where the entries of equation (61) are

13

$$14 \quad A_{11} = -2\mu k \alpha - i\omega^2 M k \quad (66)$$

15

$$16 \quad A_{12} = 2\mu k \alpha - i\omega^2 M k \quad (67)$$

17

$$18 \quad A_{13} = \mu \beta^2 - \mu k^2 + i\omega^2 M \beta \quad (68)$$

19

$$20 \quad A_{14} = \mu \beta^2 - \mu k^2 - i\omega^2 M \beta \quad (69)$$

21

$$22 \quad A_{21} = i\alpha \quad (70)$$

23

$$24 \quad A_{22} = -A_{21} \quad (71)$$

$$A_{23} = ik \quad , \quad (72)$$

$$A_{24} = A_{23} \quad , \quad (73)$$

$$A_{31} = A_{23} \exp(i\alpha a) \quad , \quad (74)$$

$$A_{32} = A_{23} \exp(-i\alpha a) \quad , \quad (75)$$

$$A_{33} = -i\beta \exp(i\beta a) \quad , \quad (76)$$

$$A_{34} = i\beta \exp(-i\beta a) \quad , \quad (77)$$

$$A_{41} = A_{21} \exp(i\alpha a) \quad , \quad (78)$$

$$A_{42} = -A_{21} \exp(-i\alpha a) \quad , \quad (79)$$

$$A_{43} = A_{23} \exp(i\beta a) \quad , \quad (80)$$

$$A_{44} = A_{23} \exp(-i\beta a) \quad , \quad (81)$$

$$x_{11} = A(k, \omega) \quad , \quad (82)$$

$$x_{21} = B(k, \omega) \quad , \quad (83)$$

$$x_{31} = C(k, \omega) , \quad (84)$$

$$x_{41} = D(k, \omega) , \quad (85)$$

$$b_{11} = 0 , \quad (86)$$

$$b_{21} = 0 , \quad (87)$$

$$b_{31} = V_0 , \quad (88)$$

and

$$b_{41} = 0 . \quad (89)$$

Using equations (67) - (89) the solution to the constants  $A(k, \omega)$ ,  $B(k, \omega)$ ,  $C(k, \omega)$ , and  $D(k, \omega)$  can be calculated at each specific wavenumber and frequency using

$$\mathbf{x} = \mathbf{A}^{-1} \mathbf{b} . \quad (90)$$

Noting that for horizontal motion,  $k = 0$ , and using the coefficients from equation (90), the transfer function between the horizontal base displacement and the horizontal mass displacement can be written as



$$T_2(\omega) = \frac{1}{R_2(\omega)} = \frac{U_x(0, b, \omega)}{V_0} = \frac{1}{\cos(k_s h) - \left(\frac{M}{\rho}\right) k_s \sin(k_s h)} \quad (91)$$

where  $T_2(\omega)$  or  $R_2(\omega)$  correspond to the data from the horizontal motion experiment.

The next step is to solve the inverse problem for horizontal motion. This involves using the data and equation (91) to estimate the shear wavenumber. Equation (91) can be rewritten as

$$f(k_s) = 0 = \cos(k_s h) - \left(\frac{M}{\rho}\right) k_s \sin(k_s h) - R_2 \quad (92)$$

It is noted that this equation is identical, except for the subscripts, to equation (60). The shear wavenumber is estimated using the same procedure that was used to estimate the dilatational wavenumber above.

The material properties can be determined from the wavenumbers. First, the dilatational and shear wavespeeds are determined using

$$c_d = \frac{\omega}{k_d} \quad (93)$$

and

$$c_s = \frac{\omega}{k_s} \quad , \quad (94)$$

respectively. The Lamé constants are calculated using equations (13) and (14) written as

$$\mu = \rho c_s^2 \quad (95)$$

and

$$\lambda = \rho c_d^2 - 2\rho c_s^2 \quad (96)$$

Poisson's ratio is then calculated using

$$\nu = \frac{\lambda}{2(\mu + \lambda)} \quad (97)$$

Young's modulus can be calculated with

$$E = \frac{2\mu(2\mu + 3\lambda)}{2(\mu + \lambda)} \quad (98)$$

and the shear modulus can be determined using

$$G \equiv \mu \quad (99)$$

The above measurement method can be simulated by means of a numerical example. Soft rubber-like material properties of the

1 test specimen are used in this simulation. The material has a  
2 Young's modulus  $E$  of  $[(1e8 - i2e7) + (5e3f - i3e2f)] \text{ N/m}^2$  where  $f$  is  
3 frequency in Hz, Poisson's ratio  $\nu$  is equal to 0.40  
4 (dimensionless), density  $\rho$  is equal to  $1200 \text{ kg/m}^3$ , and a  
5 thickness  $h$  of 0.1 m. The top mass is a 0.0254 m (1 inch) steel  
6 plate that has a mass per unit area value  $M$  of  $199 \text{ kg/m}^2$ . FIG. 4  
7 is a plot of the transfer function of the system for vertical  
8 motion and corresponds to equation (59). FIG. 5 is a plot of the  
9 transfer function of the system for horizontal motion and  
10 corresponds to equation (91). In FIGS. 4 and 5, the top plot is  
11 the magnitude in decibels and the bottom plot is the phase angle  
12 in degrees.

13 FIG. 6 is a contour plot of the absolute value of equation  
14 (60) expressed in decibels versus real dilatational wavenumber on  
15 the x axis and imaginary dilatational wavenumber on the y axis at  
16 2000 Hz. The estimated dilatational wavenumber, read directly  
17 from the plot at the location the minimum value appears and  
18 marked with an arrow, is  $27.89 + 2.61i \text{ rad/m}$ . The actual value  
19 of the dilatational wavenumber is  $27.99 + 2.60i \text{ rad/m}$ , which is  
20 slightly different from the estimated value due to the surface  
21 discretization of equation (60). FIG. 7 is a contour plot of  
22 equation (60) at 5000 Hz. At this frequency, an extraneous  
23 minimum appears on the left-hand side of the plot. However,  
24 because the real part of the wavenumber must be increasing with

1 increasing frequency, the minimum corresponding to dilatational  
2 wave propagation is located at the arrow marked spot and is equal  
3 to  $65.86 + 5.60i$  rad/m, as compared to an actual value of  $65.77 +$   
4  $5.62i$  rad/m. Again, the difference between the two values can be  
5 attributed to the discretization of the surface.

6 FIG. 8 is plot of actual (solid line) and estimated (x  
7 symbols) dilatational wavenumber versus frequency. FIG. 9 is  
8 plot of actual (solid line) and estimated (+ symbols) shear  
9 wavenumber versus frequency. In FIGS. 8 and 9, the top plot is  
10 the real part of the wavenumber and the bottom part is the  
11 imaginary part of the wavenumber. FIG. 10 is a plot of actual  
12 (solid line) and estimated (real part - x symbols, imaginary part  
13 - o symbols) Young's modulus versus frequency. FIG. 11 is a plot  
14 of actual (solid line) and estimated (real part - x symbols,  
15 imaginary part - o symbols) shear modulus versus frequency. In  
16 FIGS. 10 and 11, the imaginary part of the modulus all have a  
17 negative sign but are depicted with positive signs for plotting  
18 purposes. FIG. 12 is a plot of actual (solid line) and estimated  
19 (square symbols) of the real part of Poisson's ratio versus  
20 frequency. Because the numerical example is formulated using a  
21 Poisson's ratio that is strictly real, no imaginary component is  
22 shown in this plot. Imaginary values of Poisson's ratio are  
23 possible and have been shown to theoretically exist.

24 This invention gives the ability to estimate complex  
25 dilatational and shear wavespeeds of a material that is slab-

1 shaped and subjected to compressive forces. It also allows  
2 estimation of complex Lamé constants of a material that is slab-  
3 shaped and subjected to compressive forces. Complex Young's and  
4 shear moduli of a material that is slab-shaped and subjected to  
5 compressive forces can be estimated using this invention. The  
6 invention also allows estimation of the complex Poisson's ratio  
7 of a material that is slab-shaped and subjected to compressive  
8 forces. The advantage of this patent is that it does not require  
9 a testing configuration that has to be pressurized.

10 Obviously many modifications and variations of the present  
11 invention may become apparent in light of the above teachings.  
12 In light of the above, it is therefore understood that within the  
13 scope of the appended claims, the invention may be practiced  
14 otherwise than as specifically described.